

ATTACHMENT 3

4-1 Bridge Foundation Loads

Table 1. End Supports (Abutments) at Service Limit State

Service Limit State		M_X (kip-ft)		$V_Y(\text{kips})$		P (kips) ¹ Net	
		M_{X_total}	M_{X_perm}	V_{Y_total}	V_{Y_perm}	P_{total}	P_{perm}
Eccentricity	$M_{X max}$						
	$M_{X min}$						
	P_{total_min}						
	Controlling load combination ²						
Settlement	$M_{X max}$			_ 1			
	$M_{X min}$						
	P_{total_max}						
	Controlling load combination ³						

Table 2. End Supports (Abutments) at Strength/Construction Limit States

Strength/Const	truction Limit States	M_X (kip-ft)	V_Y (kips)	P _{total} (kips) ¹ Gross
Bearing	$M_{X min}$ $M_{X min}$ P_{old_max} Controlling load			
	combination ³ $V_{Y max}$			
Sliding	V _{Y min} P _{total_min}			
	Controlling load combination ⁴			

Notes:

- 1) Axial force (*P*) is assumed positive if compressive.
- 2) Controlling load combination is the one resulting in the highest eccentricity.
- Controlling load combination is the one resulting in the highest $q_{n,u}/q_{pn}$ or $q_{g,u}/q_R$ ratio for foundations on soil, or $q_{n,max}/q_{pn}$ or $q_{g,max}/q_R$ ratio for foundations on rock.
- 4) Controlling load combination is the one resulting in the highest "factored sliding force/factored sliding resistance" ratio.



Table 3. Intermediate Supports (Bents & Piers) at Service Limit State

Service Limit State		M_X (M_X (kip-ft)		M_Y (kips)		P (kips) ¹ Net	
		M_{X_total}	M_{X_perm}	M_{Y_total}	M_{Y_perm}	P_{total}	P_{perm}	
Eccentricity	$M_{X max}$							
	$M_{X min}$							
	$M_{Y max}$							
	$M_{Y min}$							
	P_{total_min}							
	Controlling load combination ²							
Settlement	$M_{X max}$							
	$M_{X min}$							
	$M_{Y max}$				7			
	$M_{Y min}$							
	P_{total_max}							
	Controlling load combination ³							

Table 4. Intermediate Supports (Bents & Piers) at Strength Limit State

Strengt	h Limit State	M_X (kip-ft)	M_Y (kip-ft)	V_X (kips)	V_Y (kips)	P_{total} (kips) ¹ Gross
Bearing	$M_{X max}$					
	$M_{X min}$					
	$M_{Y max}$					
	$M_{Y min}$					
	P _{total_max}					
	Controlling load					
	combination ³					
Sliding	$V_{X max}$					
	$V_{X min}$					
	V _{Y max}					
	$V_{Y min}$					
	P_{total_min}					
	Controlling load combination ⁴					

Notes:

- 1) Axial force (*P*) is assumed positive if compressive.
- 2) Controlling load combination is the one resulting in the highest eccentricity.
- 3) Controlling load combination is the one resulting in the highest $q_{n,u}/q_{pn}$ or $q_{g,u}/q_R$ ratio for foundations on soil, or $q_{n,max}/q_{pn}$ or $q_{g,max}/q_R$ ratio for foundations on rock.
- 4) Controlling load combination is the one resulting in the highest "factored sliding force/ factored sliding resistance" ratio.



Table 5. Intermediate Supports (Bents & Piers) at Extreme Event-I Limit State (Seismic)

Extreme Event–I	M (ki	p-ft)	<i>V</i> (k	ips)	P (kips)
Limit State	M_X	M_Y	V_X	V_Y	P_{Total}
M_{dl} -I ^{1,2}					
M_{d2} -I					
M_{d3} -I					
M_{d4} -I					
M_{d5} -I					
M_{d6} -I					
M_{d7} -I					
M_{dl} -II					
M_{d2} -II					
M_{d3} -II					
M_{d4} -II					
M_{d5} -II					
M_{d6} -II					
M_{d7} -II		1			

Notes:

- 1) M_{dI} - M_{d7} stand for cases where seismic overstrength moment (M_o) and associated shear (V_o) are applied at 15 degree increments $(0^{\circ}$ $90^{\circ})$ to a symmetrical spread footing.
- 2) Cases I and II correspond to highest and lowest column axial forces resulted from seismic overturning in multicolumn bents. For single-column bents the two cases (I and II) will be summarized into one case.