

| | | | | | |
|------|--------|-------|--------------------------|-----------|--------------|
| DIST | COUNTY | ROUTE | POST MILES TOTAL PROJECT | SHEET No. | TOTAL SHEETS |
| X | X | X | X | X | X |

| | |
|---------------------------|------|
| REGISTERED CIVIL ENGINEER | DATE |
| | X |

| |
|---------------------|
| PLANS APPROVAL DATE |
| |

The State of California or its officers or agents shall not be responsible for the accuracy or completeness of electronic copies of this plan sheet.

TABLE OF PILE SPACING: CLASS 45 - CONCRETE PILES

| DESIGN H | 6' | 8' | 10' | 12' | 14' | 16' | 18' | 20' | 22' | 24' | 26' | 28' | 30' | 32' |
|---------------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|
| W | 7'-3" | 7'-6" | 8'-0" | 8'-6" | 9'-6" | 10'-3" | 11'-0" | 12'-3" | 14'-0" | 14'-9" | 17'-3" | 19'-0" | 21'-3" | 22'-9" |
| F | 1'-6" | 1'-6" | 1'-6" | 1'-6" | 1'-6" | 1'-9" | 2'-0" | 2'-3" | 2'-9" | 3'-0" | 3'-3" | 3'-9" | 4'-0" | 4'-6" |
| M | 1'-0" | 1'-0" | 1'-0" | 1'-0" | 1'-0" | 1'-0" | 1'-0" | 1'-0" | 1'-0" | 1'-0" | 1'-0" | 1'-0" | 1'-0" | 1'-0" |
| N | 5'-3" | 5'-6" | 6'-0" | 6'-6" | 7'-6" | 8'-3" | 9'-0" | 10'-3" | 12'-0" | 12'-9" | 15'-3" | 17'-0" | 19'-3" | 20'-9" |
| ROW 1 | 8'-0" | 8'-0" | 8'-0" | 7'-0" | 6'-0" | 4'-6" | 4'-0" | 4'-0" | 4'-0" | 4'-0" | 4'-0" | 4'-0" | 4'-0" | 4'-0" |
| ROW 2 | 16'-0" | 16'-0" | 16'-0" | 14'-0" | 15'-0" | 11'-3" | 10'-0" | 12'-0" | 6'-0" | 4'-0" | 4'-0" | 4'-0" | 4'-0" | 4'-0" |
| ROW 3 | | | | | | | | 8'-0" | 8'-0" | 6'-0" | | | | |
| ROW 4 | | | | | | | | | | 6'-0" | 6'-0" | | | |
| ROW 5 | | | | | | | | | | | | 6'-0" | 6'-0" | |
| CONFIGURATION | I | I | I | I | I | I | I | II | II | II | III | III | III | III |

DESIGN DATA

DESIGN: LOAD FACTOR DESIGN (LFD)

CONCRETE: REINFORCED CONCRETE, $f_c = 3600$ psi
 $f_y = 60000$ psi

LOADING CASE:
LEVEL GROUND WITH 240 psf LIVE LOAD SURCHARGE AND 16' SOUNDWALL.
SEISMIC LOAD = 0.3 DEAD LOAD
WIND LOAD = 30 psf
DEAD LOAD OF SOUNDWALL = 1414 lb/lf
DEAD LOAD OF BARRIER = 372 lb/lf

SEISMIC LOAD: SOIL
 $K_h = 0.3g$
 $K_v = 0.0$
 K_{ae} : MONOBE-OKABE METHOD

SOIL: $\theta = 34^\circ$ $\gamma = 120$ pcf

EQUIVALENT FLUID PRESSURE:
= 36 pcf MAX. FOR DETERMINATION OF TOE PRESSURE
= 27 pcf MAX. FOR DETERMINATION OF HEEL PRESSURE

LOAD COMBINATIONS:
GROUP A : $\beta D + 1.7E + 1.7SC$
GROUP B : $\beta D + 1.7E + 1.3W$
GROUP C :
STEM : $1.0D + 1.0E + 1.0EQD + 1.0EQE$
FOOTING : $D + PYM$

ALTERNATIVE A: (CONSTANT TOP OF WALL THICKNESS) TABLE OF REINFORCING STEEL DIMENSIONS AND DATA

| DESIGN H | 6' | 8' | 10' | 12' | 14' | 16' | 18' | 20' | 22' | 24' | 26' | 28' | 30' | 32' |
|----------------------|---------|---------|--------|---------|---------|---------|---------|----------|---------|---------|---------|----------|---------|----------|
| STEM BATTER | 0 | 1/2:12 | 1/2:12 | 1/2:12 | 1/2:12 | 1/2:12 | 1/2:12 | 1/2:12 | 1/2:12 | 5/8:12 | 3/4:12 | 7/8:12 | 1:12 | 1:12 |
| STEM THICKNESS @ TOP | 1'-0" | 1'-0" | 1'-0" | 1'-0" | 1'-0" | 1'-0" | 1'-0" | 1'-0" | 1'-0" | 1'-0" | 1'-0" | 1'-0" | 1'-0" | 1'-0" |
| a BARS | | | | #6@18** | #7@18** | #8@18** | #9@18** | #10@18** | #8 @ 9* | #8 @ 9* | #7 @ 6* | #10@12** | #9 @ 9* | #11@12** |
| Y | | | | 8'-0" | 6'-6" | 7'-6" | 8'-0" | 9'-0" | 9'-6" | 11'-0" | 11'-6" | 13'-0" | 14'-0" | 15'-6" |
| b BARS | #5 @ 12 | #5 @ 12 | #5 @ 9 | #6@18** | #7@18** | #8@18** | #9@18** | #10@18** | #8 @ 9* | #8 @ 9* | #7 @ 6* | #10@12** | #9 @ 9* | #11@12** |
| Y | CONT | CONT | CONT | CONT | 10'-6" | 13'-0" | 15'-0" | 17'-6" | 19'-6" | 21'-0" | 18'-6" | 19'-0" | 25'-6" | 23'-6" |
| c BARS | | | | | #6 @ 18 | #6 @ 18 | #6 @ 18 | #6 @ 18 | #6 @ 18 | #6 @ 18 | #6 @ 12 | #7 @ 12 | #7 @ 18 | #7 @ 12 |
| SHORT d BARS | | | | | #8 @ 18 | #8@18** | #9@18** | #8@12** | #8@12** | #6 @ 6* | #8 @ 9* | #7 @ 6* | #9 @ 9* | #9 @ 9* |
| X | | | | | 5'-6" | 6'-6" | 6'-6" | 7'-6" | 9'-6" | 10'-0" | 12'-0" | 13'-0" | 15'-0" | 16'-0" |
| d BARS | #5 @ 12 | #5 @ 9 | #6 @ 9 | #7 @ 9 | #8@18** | #8@18** | #9@18** | #8@12** | #8@12** | #6 @ 6* | #8 @ 9* | #7 @ 6* | #9 @ 9* | #9 @ 9* |
| X | CONT | CONT | CONT | CONT | CONT | CONT | CONT | CONT | CONT | CONT | CONT | CONT | CONT | CONT |
| TOTAL e BARS | 12 #6 | 12 #6 | 10 #5 | 12 #5 | 12 #5 | 14 #5 | 14 #5 | 16 #5 | 18 #5 | 18 #5 | 20 #5 | 22 #5 | 26 #5 | 26 #5 |

ALTERNATIVE B: (VARIABLE TOP OF WALL THICKNESS) TABLE OF REINFORCING STEEL DIMENSIONS AND DATA

| DESIGN H | | 8' | 10' | 12' | 14' | 16' | 18' | 20' | 22' | 24' | 26' | 28' | 30' | 32' |
|----------------------|--|--------|--------|---------|---------|---------|---------|---------|---------|---------|---------|---------|---------|----------|
| BATTER | | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1/4:12 | 1/2:12 | 1/2:12 | 1/2:12 |
| STEM THICKNESS @ TOP | | 1'-0" | 1'-0" | 1'-0" | 1'-0" | 1'-3" | 1'-3" | 1'-6" | 2'-0" | 2'-0" | 2'-0" | 2'-0" | 2'-0" | 2'-0" |
| a BARS | | | | #6@12** | #5 @ 6* | #9@18** | #9@12** | #9@12** | #9@12** | #7 @ 6* | #7 @ 6* | #7 @ 6* | #9 @ 9* | #11@12** |
| Y | | | | 5'-6" | 5'-6" | 7'-0" | 7'-0" | 8'-0" | 8'-6" | 9'-6" | 11'-0" | 12'-6" | 13'-6" | 14'-6" |
| b BARS | | #5 @ 9 | #6 @ 9 | #6@12** | #5 @ 6* | #9@18** | #9@12** | #9@12** | #9@12** | #7 @ 6* | #7 @ 6* | #7 @ 6* | #9 @ 9* | #11@12** |
| Y | | CONT | CONT | CONT | CONT | 9'-6" | 9'-6" | 11'-0" | 11'-6" | 17'-0" | 19'-6" | 21'-6" | 24'-0" | 24'-0" |
| c BARS | | | | | | #7 @ 18 | #7 @ 12 | #7 @ 12 | #7 @ 12 | #6 @ 12 | #6 @ 12 | #6 @ 12 | #7 @ 18 | #7 @ 12 |
| SHORT d BARS | | | | | | #8@18** | #8@18** | #9@18** | #8@12** | #8@12** | #6 @ 6* | #8 @ 9* | #7 @ 6* | #9 @ 9* |
| X | | | | | | 5'-6" | 6'-6" | 6'-6" | 7'-6" | 9'-6" | 10'-0" | 12'-0" | 13'-0" | 15'-0" |
| d BARS | | #5 @ 9 | #6 @ 9 | #7 @ 9 | #8@18** | #8@18** | #9@18** | #8@12** | #8@12** | #6 @ 6* | #8 @ 9* | #7 @ 6* | #9 @ 9* | #9 @ 9* |
| X | | CONT | CONT | CONT | CONT | CONT | CONT | CONT | CONT | CONT | CONT | CONT | CONT | CONT |
| TOTAL e BARS | | 12 #6 | 10 #5 | 12 #5 | 12 #5 | 14 #5 | 14 #5 | 16 #5 | 18 #5 | 18 #5 | 20 #5 | 22 #5 | 26 #5 | 26 #5 |

CONT = CONTINUOUS

* = a AND b [SHORT d & d] BARS ARE BUNDLED TOGETHER.

** = ALTERNATE a AND b BARS AS SHOWN IN DETAIL A.

ALTERNATE SHORT d AND d BARS.

WHERE : $\beta = 1.0$ OR 1.3 WHICHEVER CONTROLS DESIGN
D = DEAD LOAD
E = LATERAL EARTH PRESSURE
SC = LIVE LOAD SURCHARGE
W = WIND LOAD
EQD = SEISMIC DEAD LOAD
EQE = SEISMIC LATERAL EARTH PRESSURE

- GENERAL NOTES**
- CLASS 45-CONCRETE PILES WERE USED FOR THE DESIGN.
 - PILE BATTER SHOWN ARE 1:3.
 - MINIMUM DISTANCE BETWEEN CENTER PILE AND EDGE OF FOOTING IS 1'-6".
 - REDUCTION FACTORS:
GROUPS A & B : $\theta = 0.75$
GROUP C : $\theta = 1.0$
 - LATERAL RESISTANCE OF EACH PILE:
GROUPS A & B : = 30 kip
GROUP C : = 40 kip
 - MAXIMUM SPACING BETWEEN PILES IS SHOWN IN THE TABLE. REDUCE TO SUIT THE LENGTH OF FOOTING.
 - MINIMUM DISTANCE BETWEEN ANY TWO PILES IS 3'-0".

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|-------------------------------------------------|----------------------------|-------------------------------------|--|-----------------------------------------|--|---------------------|--|
| STANDARD DRAWING | | STATE OF CALIFORNIA | | DIVISION OF ENGINEERING SERVICES | | BRIDGE NO. X | |
| FILE NO. xs14-360-1x | APPROVAL DATE X-X-X | DEPARTMENT OF TRANSPORTATION | | PROJECT NUMBER & PHASE: X | | POST MILE X | |
| RETAINING WALL TYPE 5SWP - DETAILS NO. 1 | | | | CONTRACT NO.: X | | SHEET X OF X | |

DISREGARD PRINTS BEARING EARLIER REVISION DATES

REVISION DATES

DATE PLOTTED => \$DATE USERNAME => \$USER